## NEW SOUTH WALES

## DEVELOPMENT CONSTRUCTION SPECIFICATION

## C402

# SEWERAGE SYSTEM

**VERSION 3.0** 

### **SPECIFICATION C402 - SEWERAGE SYSTEM**

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### Amendment Record for this Specification Part

This specification is Council's edition of the AUS-SPEC generic specification part and includes Council's primary amendments.

Details are provided below outlining the clauses amended from the Council edition of this AUS-SPEC Specification part. The clause numbering and context of each clause are preserved. New clauses are added towards the rear of the specification part as special requirements clauses. Project specific additional script is shown in the specification as italic font.

The amendment code indicated below is 'A' for additional script, 'M' for modification to script, and 'O' for omission of script. An additional code 'P' is included when the amendment is project specific.

Amendment Sequence No.	Key tolic addressed in amendment	Clause No.	Amendment Code	Author Initials	Amendment Date
DRAFT	Major revision for use in Northern Rivers Local Government Manuals	All	AMOP	GAK	30/3/2009
VERSION 3.0	Minor changes to DRAFT following consultation with Councils	Various	AMO	GAK	15/5/2009

### **DEVELOPMENT CONSTRUCTION SPECIFICATION C402**

### SEWERAGE SYSTEM

### GENERAL

### C402.01 SCOPE

- 1. This specification is for construction of:
  - (a) Gravitation sewers up to DN600 nominal size;
  - (b) Common Effluent sewers, both gravity and pressurised;
  - (c) Vacuum Sewerage Systems;
  - (d) Rising mains up to DN600 nominal size;
  - (e) Standard appurtenances such as maintenance holes, maintenance shafts and property connection sewers;
  - (f) Small pump stations, usually limited to single wells with submersible pumps.
- 2. This Specification excludes the construction activities for:
  - (a) Treatment plants;
  - (b) Headworks;
  - (c) Dosing plant;
  - (d) Larger pump stations;
  - (e) Works controlled by others, including overflow management
- 3. The Contractor shall carry out the work, and supply materials meeting the requirements of the reference documents and, in particular, in accordance with the requirements of the Water Services Association of Australia's publication WSA 02 SEWERAGE CODE OF AUSTRALIA, WSA 04 SEWAGE PUMPING STATION CODE OF AUSTRALIA, WSA 06 VACUUM SEWERAGE CODE OF AUSTRALIA, and WSA 07 PRESSURE SEWERAGE CODE OF AUSTRALIA unless specified otherwise herein. Sewerage works should be designed in accordance with the DEVELOPMENT DESIGN SPECIFICATION D12 SEWERAGE SYSTEM.
- 4. For the purposes of this Specification, 'access chambers' are referred to as **Terminology** 'maintenance holes'.

### C402.02 REFERENCE DOCUMENTS

1. Documents referenced in this Specification are listed below whilst being cited in **Documents** the text in the abbreviated form or code indicated. The Contractor shall possess, or have access to, the documents required to comply with this Specification.

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Suitable Works

Exclusions

 Where parallel sections or equivalent clauses to the reference of a Water Services Association of Australia's publications is called up as part of this Specification, these references are identified by part and section numbers and enclosed in brackets thus (WSA Edition, Part, Section).

### (a) Council Specifications

Northern Rivers Local Government Development and Design Manual Northern Rivers Local Government Construction Manual Northern Rivers Local Government Standard Drawings

### (b) Australian Standards

References in this Specification or on the design plans to Australian Standards are noted by their prefix AS or AS/NZS. *Standards* 

Where not otherwise specified in this Specification or the design plans, the Contractor shall use the latest Australian Standard, including amendments and supplements, available within two (2) weeks of close of tenders.

AS/NZS 1111 AS/NZS 1112		ISO metric hexagon commercial bolts and screws ISO metric hexagon nuts, including thin nuts, slotted nuts,
AS 1150		and castle nuts
AS 1152 AS/NZS 1260	-	Specification for test sieves
AS/125 1260 AS 1272	-	PVC pipes and fittings for drain, waste and vent applications Unsintered PTFE tape for thread sealing applications
AS 1272 AS 1289.5.4.1	-	Compaction control test – Dry density ratio, moisture
AS 1209.5.4.1	-	variation and moisture ratio
AS 1289.5.7.1	-	Compaction control test (-Rapid Method)
AS 1349	-	Bourdon tube pressure and vacuum gauges
AS 1444	-	Wrought alloy steels – Standard, hardenability (H) series
		and hardened and tempered to designated mechanical
		properties
AS/NZS 1477	-	PVC pipes and fittings for pressure applications
AS 1565	-	Copper and copper alloys – Ingots and castings
AS 1579	-	Arc welded steel pipes and fittings for water and wastewater
AS/NZS 1594	-	Hot-rolled steel flat products
AS 1627.4	-	Metal finishing – Preparation and pre-treatment of surfaces-
		Abrasive blast cleaning
AS 1646	-	Elastomeric seals for waterworks purposes
AS 1657	-	Fixed Platforms, walkways, stairways and ladders – Design,
		construction and installation
AS 1741	-	Vitrified clay pipes and fittings with flexible joints – sewer
		quality
AS 1830	-	Grey cast iron
AS 1939	-	Degrees of protection provided by enclosures for electrical equipment
AS 2032	-	Code of practice for installation of uPVC pipe systems.
AS 2032	-	Installation of polyethylene pipe systems
AS 2129	-	Flanges for pipes, valves and fittings
AS/NZS 2280	-	Ductile iron pressure pipes and fittings
AS 2528	-	Bolts, studbolts and nuts for flanges and other high and low
		temperature applications
AS/NZS 2566.1	1 -	Buried flexible pipelines – Structural design
AS 2837	-	Wrought alloy steels – Stainless steel bars and semi-
		finished products
AS/NZS 3000	-	Electrical installations (Wiring Rules)
AS/NZS 3008	-	Electrical installations –Selection of cables
AS 3439	-	Low-voltage switchgear and controlgear assemblies

AS 3518	-	Acrylonitrile butadiene styrene (ABS) pipes and fittings for
AS 3571	-	pressure applications Glass filament reinforced thermosetting plastics (GRP) pipes – Polyester based – Water supply, sewerage and drainage applications
AS 3578	-	Cast iron non-return valves for general purposes
AS 3681	-	Guidelines for the application of polyethylene sleeving to ductile iron pipelines and fittings
AS 3690	-	Installation of ABS pipe systems
AS 3972	-	Portland and blended cements
AS 3996	-	Metal access covers, road grates and frames
AS 4058	-	Precast concrete pipes (pressure and non-pressure)
AS 4060	-	Loads on buried vitrified clay pipes
AS 4087	-	Metallic flanges for waterworks purposes
AS/NZS 4129	-	Fittings for polyethylene (PE) pipes for pressure applications
AS/NZS 4130	-	Polyethylene (PE) pipes for pressure applications
AS 4198	-	Precast concrete access chambers for sewerage
		applications (Read 'maintenance hole' for 'access chamber')
AS/NZS 4321	-	Fusion-bonded medium-density polyethylene coating and lining for pipes and fittings
AS/NZS 4680	-	Hot-dip galvanised (zinc) coatings on fabricated ferrous articles
AS/NZS 4765(Int)		Modified PVC (PVC-M) pipes for pressure applications
AS 4794	-	Non return valves – Swing check and tilting disc

### (c) Other

Institute of Public Works Engineering Australia (IPWEA)

- Streets Opening Conference Information Bulletin on Codes and Practices (Sections 3 and 4 detailing locations and depths of other services and preferred location for water reticulation pipes)

**NSW Department of Commerce** 

MEW E101	-	Electrical Services Minimum Requirements
WS-SPEC	-	Technical Requirements (TRs) and Strategic products
		Specifications

02

WSA 02 – SEWERAGE CODE OF AUSTRALIA WSA 04 – SEWAGE PUMPING STATION CODE OF AUSTRALIA WSA 06 – VACUUM SEWERAGE CODE OF AUSTRALIA WSA 07 – PRESSURE SEWERAGE CODE OF AUSTRALIA

British Standard BS 410 - Specification for test sieves

### (d) Standard Drawings that apply to this section;

### Drawings

It is intended to develop a series of standard drawings for inclusion in the Northern Rivers Local Government Standard Drawings relating to water supply and sewerage systems. When these are developed, these drawings will be used in preference to other standard drawings. Where there is not a suitable standard drawing included in the Northern Rivers Local Government Standard Drawings, Council will consider use of other standard drawings.

Other standard drawings may be used, subject to assessment by the individual Council. Where proposed to use other standard drawings, such as those listed below, copies are to be provided with each set of design drawings to allow the use of the standard drawing to be assessed by the individual Council.

- > Tweed Shire Council Standard Drawings
- IPWEA Standard Drawings
- WSA Standard Drawings

### MATERIALS

### C402.03 GENERAL

1.	The Contractor shall comply with the requirements of the manufacturer's recommendations regarding the handling, transport and storage of materials and as further specified in this Specification.	Due Diligence			
2.	The Contractor shall not use damaged or defective materials, including coatings and linings, outside the manufacturer's recommended limits.	Rejection			
3.	All gravity reticulation pipes shall be rubber ring (elastomeric), complying with AS 1646, jointed to the type, size and class as shown on the design plans.	Pipes			
C402.0	4 UNPLASTICISED AND MODIFIED PVC (uPVC and PVC-M) PIPE AND FITTINGS				
1.	Unplasticised PVC (uPVC) pipes and fittings for gravity systems shall comply with AS/NZS 1260, shall be suitable for rubber rings (elastomeric) joints and shall be of the class and size as shown on the design plans. (WSA 02, 2, 10).	Non-pressure Pipe PVC			
2.	Unplasticised PVC (uPVC) pipes and fittings for rising mains and suction pipes shall comply with AS/NZS 1477 and AS/NZS 4765, shall be suitable for rubber ring (elastomeric) joints shall be of the class and size as shown on the design plans. Modified PVC (PVC-M) pipes and fittings shall comply with AS/NZS 4765, shall be suitable for rubber ring (elastomeric) joints and shall be of the class and size as shown on the design plans.	Pressure Pipe PVC			
3.	PVC pipes and fittings for mains and suction pipes shall be installed in accordance with AS 2032 and AS/NZS 2566.1.	Installation			
4.	Pipes and fittings are to be handled and stored protected from sunlight. The Contractor shall provide protection for the pipes and fittings from ultra violet light and damage. The Contractor shall take account of the time for storage and type of shelter.	Protection			
C402.05 POLYETHYLENE (PE) PIPE AND FITTINGS					
1.	Polyethylene pipe shall comply with AS/NZS 4129 and AS/NZS 4130 and shall be of the class and size shown on the design plans and installed in accordance with AS 2033. (WSA 02, 2, 10).	Standard			
2.	Jointing shall be by butt thermal fusion or by electrofusion couplings, or with compression fittings.	Jointing			
3.	The Contractor shall provide pipe of the appropriate external diameter consistent with the required internal diameter shown on the design plans.	Internal Diameter			

### C402.06 GLASS REINFORCED PLASTIC (GRP) PIPE AND FITTINGS

- 1. Glass filament reinforced thermosetting plastics (GRP) pipes shall comply with AS 3571 and shall be of the class and size as shown on the design plans and installed in accordance with AS/NZS 2566.1. (WSA 02, 2, 10).
- 2. Pipes and fittings are to be handled and stored protected from sunlight. The Contractor shall provide protection for the pipes and fittings from ultra violet light and damage. The Contractor shall take account of the time for storage and type of cover.

### C402.07 DUCTILE IRON (DI) PIPE AND FITTINGS

- 1. Ductile iron (DI) pipes and fittings shall comply with AS/NZS 2280 and shall be of the class, size and lining, as shown on the design plans, and installed in accordance with AS/NZS 2566.1. Jointing shall be with rubber rings (elastomeric) to the class and type as shown on the design plans. (WSA 02, 2, 10)
- Flanges shall be to the table shown on the design plans. Bolts and nuts for flanged joints shall be stainless steel, unless shown otherwise on the design plans.
- 3. All pipework shall be sleeved externally with polyethylene sleeving in accordance with the requirements of AS 3681 unless specified otherwise to be coated and lined. All fittings shall be fusion-bonded coated, in accordance with AS/NZS 4321, or wrapped. The Contractor shall wrap all unprotected joints in the trench with a petrolatum tape system approved by Council.

### C402.08 STEEL PIPELINE

- 1. Steel pipelines and fittings shall comply with AS 1579 and AS/NZS 1594 and shall be of the class, size, lining and coating as shown on the design plans. (WSA 02, 2, 10).
- 2 The Contractor shall wrap all unprotected joints in the trench with a petrolatum *Corrosion* tape system approved by Council. *Protection*
- 3. The jointing system shall be rubber ring (elastomeric) unless shown otherwise on **Joints** the design plans.

### C402.09 DELETED

### C402.10 PREFORMED MAINTENANCE HOLES (MH)

 Preformed maintenance hole components shall comply with AS/NZS 1477 for PVC, AS 2033 for PE, AS 3518 for ABS, AS 3571 for GRP and AS 4198 for concrete. (WSA 02, 1, 6 & 3, 18)

### C402.11 PREFORMED MAINTENANCE SHAFTS (MS) AND TERMINAL MAINTENANCE SHAFTS (TMS) INCLUDING COVER

 Preformed maintenance shaft and terminal maintenance shaft components shall comply with AS/NZS 1477 for PVC, AS 2033 for PE, AS 3518 for ABS, AS 3571 for GRP and AS 4198 for concrete. (WSA 02, 1, 6 & 3, 19, SEW-1314, SEW-1316)

### C402.12 MAINTENANCE HOLE COVERS AND FRAMES

- Maintenance hole covers and frames shall comply with AS 3996 and shall be solid top, marked with SAN-SEW, size and class as shown on the design plans. Frames shall be standard width of 600m diameter or 600mm x 900mm opening and shall be ductile iron Grade 600/B in accordance with AS 1831.
- 2. Concrete covers and frames shall comply with AS 4198 and shall be of the size **Concrete** and, either Heavy or Light, class as shown on the design plans.

### C402.13 STEELWORK

- 1. Structural steelwork, including ladders, brackets and covers, complying with AS 1657, shall be abrasive blast cleaned to AS 1627.4, Class 2.5 and hot dip *Protection* galvanised to AS/NZS 4680.
- Step irons are not generally required, but the Contractor shall supply and install plastic encapsulated step irons when shown on the design plans. (WSA 02 Part 4, section 4.22).

### **PIPELINE CONSTRUCTION**

### C402.14 GENERAL

1. The Contractor shall not change the pipeline alignment without the prior concurrence of Council. The Contractor shall provide full details, of any proposed changes to the pipeline alignment to Council for consideration. This action constitutes a **HOLD POINT**. The Contractor shall obtain the decision of Council prior to the release of the hold point.

### C402.15 LOCATION

1. The location of the sewers, maintenance holes, rising mains and pump stations, sizes and grades of sewers and rising mains, the types of maintenance holes and maintenance hole covers and the classes of pipes shall be as shown on the design plans. The Contractor shall commence laying of pipelines at the lower end of the line unless directed otherwise by Council. The Contractor shall lay pipelines to grades and locations shown on the design plans unless directed otherwise by Council. (WSA 02, 3, 13.1 & 13.2).

### C402.16 COVER OVER PIPELINES

1. The minimum depth of cover to be provided over pipelines shall be as follows: (WSA 02, 3, 15.2).

tions, *Pipe Laying* holes *Method* 

Alignment

HP

Changes

Cover

Minimum

LOCATION	MINIMUM COVER (mm)
Private property non vehicular	
New Developments	600
Private property non vehicular	
Existing Developments	450
Private property vehicular	
	750
Footpaths, sealed roads (non Arterial)	
	900
Unsealed roads	
	1200
Arterial roads	
	1200

2. Lesser covers may be permitted where special protection of the pipelines has been shown on the design plans or directed by Council.

### C402.17 CROSSINGS

- 1. Where a pipeline crosses a Main or State road, creek or involves features shown on the design plans, under the control of any other Authority, the Contractor shall carry out the work in accordance with the requirements of that Authority. The Contractor shall provide written notification to the Authority of the intention to carry out the work, and pay the appropriate fees (WSA 02, 3, 17.13). The Contractor shall obtain the written approval from the Authority prior to commencement of work. Such written approval shall be supplied to Council if requested. This action constitutes a **WITNESS POINT**. Council shall advise at the time of notification by the Contractor whether the option to request the written approval is to be exercised.
- Where shown on the design plans, the Contractor shall use trenchless methods for the installation of the sewer mains. The installation of the sewer main by open trenching shall not be permitted over the lengths designated for trenchless installation. (WSA 02, 3, 13.5.4 & 17.12).
- 3. The Contractor shall address, in its Method Statement for trenchless conduit installation, the following:-
  - (a) General description of method and sequence of operation.
  - (b) Size, depth and position of temporary pits required.
  - (c) Use of specialist subcontractors.
  - (d) Specialist equipment to be used.
  - (e) Grout type and method of injection.
- 4. The encasement pipe shall be as detailed on the design plans. The encasement **Encasement** pipe shall extend 1.0m behind the back of the kerb or drainage system on either **Pipe** side of the carriageway.
- 5. The carrier pipe shall be positioned on support cradles and the carrier pipe shall **Support Cradles**

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Special

Protection

Contractor's

Responsibility

WP

Trenchless

Installation Methodology

- 6. After installation and pressure testing of the carrier pipe, the Contractor shall fill **Grouting** the annular space between the carrier pipe and the encasement pipe with suitable grout or cementitious grout filler. (WSA 02, 3, 17.12)
- 7. Where the carrier pipe is ductile iron cement lined (DICL), any length of pipe which is enclosed within the encasement pipe need not be wrapped in polyethylene tubing.

### C402.18 EARTHWORKS

- The Contractor shall carry out all excavations for structures and pipelines to the lines, grades and forms shown on the design plans, or as directed by Council, within the specified tolerances. The Contractor shall comply with all requirements of the appropriate Authority including having regard for drainage, dewatering, silt control, noise abatement, proximity to existing buildings and generally for the amenity of adjacent owners. (WSA 02, 3, 15).
- The Contractor shall leave a clear space of 600mm minimum between the edge of any excavation and the inner toe of stockpiles. No excavated materials shall be stockpiled against the walls of any building or fence without the written permission of the owner of such building or fence. Topsoil from excavations shall be stockpiled separately and utilised to restore the surface after backfilling. (WSA 02, 3, 14.7 & 15.9).
- 3. Whenever open excavations are located in a public area and/or are left unattended, the Contractor shall install safety fencing to Statutory requirements along the edges of open excavations to isolate them from the public. The Contractor shall provide fenced walkways and vehicular crossways across trenches to maintain access at all times from carriageway to individual properties or within individual properties and advise beforehand all affected residents. All such installations shall be of adequate size and strength and shall be illuminated to prevent accidents. (WSA 02, 3, 15.1)
- 4. The Contractor shall locate, protect and repair, as necessary, all services *Existing* affected by the Works at the Contractor's expense. (WSA 02, 3, 13.5.2 & 13.7) *Services*
- 5. The Contractor shall carry out erosion and sedimentation control at all *Erosion* construction sites. *Control*
- 6. The Contractor shall take account of safety issues and possible wet weather effects to limit the extent of excavation left open. (WSA 02, 3, 15.2)

### C402.19 MINIMUM TRENCH WIDTH FOR PIPELINES

1. The minimum clear width of trench (inside internal faces of timbering or sheet piling, if used) to a height of 150mm above the top of the pipe shall be as shown in Table C402.1. (WSA 02, 3, 15.2).

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Limiting

Excavations

NOMINAL SIZE OF PIPE (DN)	MINIMUM CLEAR WIDTH OF TRENCH (mm) (inside timbering or sheet piling, if any)		
	PIPE OTHER THAN PVC/PE	PVC/PE PIPE	
80	400	350	
100	400	350	
150	450	400	
200	500	450	
225	550	500	
250	550	500	
300	600	550	
375	700	650	
400	700	650	
450	750	700	
500	850	800	
525	850	800	
600	950	900	

Table C402.1	-	Minimum	Trench	Widths
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2.	Where the design plans provide for a trench to be excavated across a paved surface, the width of the trench shall be kept to a minimum. Bitumen and concrete surfaces shall be carefully cut, by sawcutting, or other means approved by Council, so as to provide a neat straight line free from broken ragged edges. (WSA 02, 3, 15.3)	Minimum Disturbance
3.	The Contractor shall widen the trench where necessary for the installation of valves and fittings and protective coating systems.	Widen For Fittings
C402.2	20 MAXIMUM TRENCH WIDTH	
1.	For gravitation sewers or rising mains of pipe materials other than PVC or PE, no restriction shall be placed on the maximum width of trench due to the structural strength of the pipe provided the depth to invert of the pipe does not exceed the depths shown in column (ii) of Table C402.2.	Pipes other than PVC/PE
2.	Council may, however, restrict the width of trench due to local conditions. Council shall not restrict the width of trench to less than as shown in column (iii) of Table C402.2.	Width Restrictions
3.	Where the depth to invert exceeds that shown in column (ii) of Table C402.2, the maximum width of trench (outside timbering or sheet piling, if used) to a height of 150mm above the top of the pipe shall be as shown in column (iii) of Table C402.2.	Depth

Nominal Size of Pipe (mm) (i)	Maximum Depth to Invert, Unlimited Width Trench (m) (ii)	Maximum Trench Width, Depths Greater than in Column (ii) (mm) (iii)
150	8.0	750
225	6.5	825
300	5.5	900
375	4.5	975
400	4.5	975
450	4.5	1050
525	4.0	1125
600	4.0	1200

### Table C402.2 - Maximum Trench Widths

- For gravitation sewers or rising mains of PVC/PE pipe the maximum width of 4 **PVC/PE** Pipe trench from the trench base to a height of 150mm above the top of the pipe shall be the outside diameter of the pipe barrel plus 400mm. However, in timbered or travelling box excavated trenches, the width of trench when measured to the outside of the support used may be increased to a maximum of 580mm plus the outside diameter of the pipe barrel.
- 5. The Contractor shall supply a method statement of any special construction Special control, where shown on the design plans, and this will be subject to Council's **Controls** approval.

#### **EXCAVATION DEPTH** C402.21

- The Contractor shall excavate trenches to 75mm below the underside of the pipe 1. 75mm Below barrel and socket or coupling except for rising mains to be laid on other than rock foundations or as otherwise shown on the design plans. (WSA 02, 3, 15.8)
- 2. The excavation shall be carried out such as to ensure solid and uniform support Pipe Support for each pipe over the whole length of the barrel with chases provided for joints and wrapping.

#### SUPPORT OF EXCAVATION C402.22

- The Contractor shall adequately support all excavations to Statutory 1. Precaution requirements as the Works proceed. When withdrawing supports, the Contractor Against Slips shall exercise every precaution against slips or falls. (WSA 02, 3, 15.6). or Falls
- 2. The Contractor shall ensure that timber is left in place where its removal may endanger structures in the vicinity of the excavation.

#### C402.23 **PIPE BEDDING**

- When excavation of the trench has been completed the Contractor shall obtain 1. Council's approval prior to commencing pipe laying, jointing and bedding. This action constitutes a HOLD POINT. Council's approval of the excavated trench is required prior to the release of the hold point.
- Crusher screenings may only be used for pipe bedding where sand or other non-2. cohesive material is not readily available locally or where the Contractor can demonstrate that its use will not impede repair operations. (WSA 02, 3, 16).
- 3. Pipes for gravitation sewers (excluding PVC/PE pipes), shall be bedded on sand or other non-cohesive material. Pipe bedding shall consist of a non-cohesive © The AUS-SPEC Joint Venture date: Jan 2002 Copying for on selling strictly prohibited

Approval

Place

Timber Left in

HP

Crusher

Screenings

Gravity Sewers granular material, having a minimum thickness of 75mm below the barrel and socket of the pipe, and its grading shall generally fall within the following limits shown in Table C402.3.

Pipes other than PVC/PE

**Rising Mains** 

**Pipes other** 

than PVC/PE

Sieve Size Aperture Width (AS1152)	Equivalent BS Sieve Size (BS410)	Percentage Passing
22.4 mm	1 inch	100
6.7 mm	¼ inch	90 - 100
425 μm	No. 36	40 - 90
75 μm	No. 200	0 - 10

### Table C402.3 - Grading of Bedding Material for Pipes Other Than PVC and PE

- 4. Pipes for rising mains (excluding PVC/PE pipes) may be laid directly on other than rock foundation. The Contractor shall provide non-cohesive granular bedding, having a minimum thickness of 75mm below the barrel and socket of the pipe, where rock or other hard material occurs in the bottom of the trench or where specified or directed by Council. The bedding material shall be either loose clean sand and /or medium dense clean sand or as directed by Council.
- 5. For PVC/PE pipes, irrespective of foundation, the material to be used for pipe bedding (underlay a minimum of 75mm below the underside of the pipe barrel and socket, side support and overlay to a depth of 150mm above the top of the pipe) as shown in Figure 5.1 in AS 2032 shall be in sand or other non-cohesive granular material, either crushed, natural or blended, and its grading shall fall within limits shown in Table C402.4, except that where the materials cannot be reasonably sourced from within the vicinity, the Contractor may use materials satisfying the classification in paragraph 2 above provided also that the material meets the requirements for passing sieve sizes 9.5mm and 6.7mm as shown in Table C402.4.

Sieve Size Aperture Width (AS1152)	Equivalent BS Sieve Size (BS410)	Percentage Passing
9.5 mm	<sup>3</sup> / <sub>8</sub> inch	100
6.7 mm	1⁄4 inch	90 - 100
425 μm	No. 36	40 - 90
150 μm	No. 100	0 - 10

### Table C402.4 Grading of Bedding Material for PVC and PE Pipes

- 6. The Contractor shall bed all gravitation sewers laid on grades of 15 per cent to 50 per cent on 20MPa concrete complying with C271 Minor Concrete Works. Such concrete bedding shall have a thickness of at least 75mm below the underside of the barrel and socket of the pipe and shall extend to a level above the bottom of the pipe of one quarter of the external diameter of the pipe and a width across the trench not less than the minimum width shown in Table C402.1. (DPWS WG.ST520A).
- 7. The Contractor shall encase all gravitation pipelines and rising mains, laid on grades steeper than 50 per cent, in concrete as detailed on the design plans.

15-50% Grades

Grades Greater Than 50%

### C402.24 LAYING AND JOINTING OF PIPES

- Unless detailed otherwise in this Specification, the Contractor shall install pipes 1. Installation in accordance with AS 2032, AS 2033, AS/NZS 2566.1 or AS 3690 as appropriate. (WSA 02, 3, 17). Before being laid, all pipes, fittings, valves, and materials to be used shall be 2. Examination cleaned and examined by the Contractor and, if required by Council, the Contractor shall suspend each one in a sling to enable Council to inspect it. If directed by Council, the Contractor shall oil valves and repack valve glands. 3. The Contractor shall ensure that the interior of the pipeline is clean and free from Cleaning obstructions. Plugs shall be used to prevent foreign matter entering sections of pipeline which are left uncompleted overnight. 4. The Contractor shall take all necessary precautions to prevent flotation of pipes Flotation during laying, backfilling and initial testing. The Contractor shall remove any temporary supports prior to completion of backfilling. Except where solvent cement joints are needed to make up or install fittings, 5. Joint Type joints in pipelines shall be flexible, rubber ring (elastomeric) joints (either roll-on rubber ring (elastomeric) or skid type) or, where shown on the design plans, mechanical joints (either fixed flange or bolted gland type). (WSA 02, 3, 17.1.2) 6. For pipes with roll-on rubber ring (elastomeric) joints, spigots and sockets shall Roll-on be clean and dry. The Contractor, after making the joint, shall check that the Rubber Ring rubber ring (elastomeric) has rolled in evenly, and, if not, the Contractor shall withdraw the pipe and remake the joint. For pipes with skid type rubber ring (elastomeric) joints, only the lubricant 7. Skid Rubber specified in writing by the manufacturer shall be applied in making the joint. The Rina Contractor shall make the joint such that the witness mark shall, at no point, be more than 1mm from the end of the socket. 8. Pipes may be cut as needed, or directed by Council, to suit closing lengths, to **Cut Pipes** remove damaged pipe or fittings or to remove sockets if necessary when jointing a socketed fitting. 9. For field cuts, a mechanical pipe cutter shall be used, except that PVC/PE pipes Pipe Cutting may be cut using a power saw or a fine toothed handsaw and mitre box. For field cuts of ductile iron or steel, the Contractor shall ensure that fire-fighting equipment, in working order, is on the site prior to the field cuts being made. If
- 10. The Contractor shall prepare the ends of any pipes cut in the field to the manufacturer's written instructions, or as directed by Council.

the Contractor proposes to use a petrol-engine pipe cutter in an excavation, the Contractor shall ensure that a safe atmosphere is maintained in the excavation at

11. Where pipes are cut in the field, the Contractor shall make a witness mark on the pipe at the length specified by the manufacturer from the end of the pipe. The Contractor shall not use PVC/PE pipes with scored witness marks. Where the same manufacturer does not make spigots and sockets, the Contractor shall refer to the socket manufacturer for the correct marking depth.

End Preparation

Witness Mark

all times.

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12.		PVC pipes are to be joined to pipes of another material, the joints shall be as follows:	Different Joints
	(a)	For jointing PVC/PE spigot to VC socket or PVC/PE socket to VC spigot, the Contractor shall use a PVC/PE adaptor shall be used. The joints in both instances shall be made using a ring conforming to AS 1646.	
	(b)	For jointing PVC/PE to ductile iron, the Contractor shall use a rubber ring (elastomeric) joint with an adaptor coupling.	
13.		Contractor shall conform with the relevant Statutory and OH&S ments when cutting and disposing of asbestos cement pipes.	Existing AC Pipe
14.	Gravita	ation pipelines shall be constructed to the following tolerances:	Tolerances
	(a)	The maximum horizontal deviations to either side from the design axis of a pipeline shall be 20mm for all sizes of pipes. (WSA 02, 3, 23.1)	
	(b)	The invert level shall not deviate from the design grade line by more than 10mm. (WSA 02, 3,23.2)	
15.	with th comply deflect	y jointed pipelines with gradual changes in alignment or grade shall be laid be joint being deflected after it has been made. The Contractor shall with the manufacturer's written recommendations in respect of maximum ion for each joint provided that no joint shall be deflected to such an extent inpair its effectiveness.	Joint Deflection
16.		aximum angle of deflection between adjacent pipes shall be limited to 2° or radian in areas subject to mine subsidence or slippage.	Limit of Joint Deflection
17.	mains	otherwise directed by Council, the Contractor shall lay pipes for rising on continuously rising grades from scour valve to air release valve, istanding any minor irregularities in the ground surface.	Rising Main Grade
18.	line of materia	detectable identification tape to AS/NZS 2648.1 shall be laid along the the rising main, positioned at either the interface between the bedding al and the backfill material, or 150mm above the top of the service when ckfill material is the same as the bedding material. (WSA 02, 3, 17.11.2)	Rising Main Identification
19.		b backfilling and compaction operations, the Contractor shall undertake tests of all pipelines for any abnormalities in pipe shape and rectify any	Ovality Testing
	unsatis	factory sections found to the satisfaction of Council. The test results of	WP
	WITNE	ests shall be made available to Council. This action constitutes a <b>SS POINT</b> . Council shall advise at the time of notification by the	
	Contra	ctor whether the option to inspect the test results is required.	

### C402.25 CONNECTIONS TO MAINTENANCE HOLES AND STRUCTURES

1. The Contractor shall connect pipelines to maintenance holes, structures or embedded concrete by means of 600mm long pipes such that two (2) flexible joints are provided, the first joint being at or within 150mm of the face of the structure. Where flexible joints cannot be made with cut pipes, the Contractor shall select pipes from the various lengths provided in order to make the second joint within 300mm of the position shown on the design plans. (WSA 02, 3, 18.10).

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Flexible Joints

Work on Live

Maintenance

**Holes** 

- 2. The Contractor may vary slightly the positions of maintenance holes shown on the design plans, subject to final approval by Council immediately prior to construction, to suit changes, such as erection of structures, growth of flora and installation of services. The positioning of a maintenance hole shall be such as to comply with occupational health and safety requirements for access by maintenance staff, providing a proper working area around the top and access into the hole. Once the final position of a maintenance hole has been established, construction shall be subject to the following requirements:
  - (a) For deviations from the design levels of maintenance holes as shown on the design plans or as directed by Council during construction, the following tolerances shall apply: (WSA 02, 3, 23).
    - (i) Where the difference in levels between the inlet pipe and the outlet pipe in a maintenance hole is 100mm or less:

Pipe	Tolerance
Inlet Outlet	- nil; + 10mm - 10mm; + nil
(ii)	Where the difference in levels, as above, is greater than 100mm:
Pipe	Tolerance
Inlet Outlet	- 10mm; + 10mm - 10mm; + 10mm

- (b) Allowable lateral deviations from the final design position of maintenance holes shall be +/- 300 mm.
- 3. The Contractor shall complete all necessary Works on "live" maintenance holes (that is, accesses to sewer system that is currently in service) unless shown otherwise on the design plans or advised by Council. Where shown on the design plans that work on "live" maintenance holes shall be performed by others, the Contractor must co-ordinate the Works with any simultaneous and/or adjacent work by others. The Contractor shall liaise with these Contractors and Authorities to avoid disruption, delays and possible conflict. All Works undertaken by the Contractor at "live" maintenance holes in completing the subdivision works shall be under supervision of Sewer Authority and at the cost to the Contractor. (WSA 02, 3, 24).

### C402.26 JUNCTIONS AND PROPERTY CONNECTION SEWERS

The Contractor shall provide junctions for dead ends and property connection sewers or risers to properties to serve existing and future dwellings in accordance with this Specification and the design plans. Such junctions shall be inserted along pipelines in locations shown on the design plans or directed by Council, with the service connection, where not shown on the design plans, provided at a depth no deeper than 1.5m provided the property still has service to the sewer, as follows: (WSA 02, 3, 17.7 & 17.8)

(a) For existing dwellings, at the most practical point not outside the property boundary to facilitate the connection, considering existing sewage outlets. Separate connections shall be provided for dual occupancies.

(b)	For vacant blocks, at the most practical point not outside the property
	boundary to facilitate the connection, considering topography and likely
	positioning of sewage outlets.

- Where the sewer is intended to serve a large block and/or where the sewer line is located more than 75m from the premises, the Contractor shall extend the property connection sewer onto that block such that the maximum horizontal measurement in a straight line between the sewer connection point and the premises on the block is not more than 75m.
- 3. Junctions for risers shall be encased in 20MPa concrete complying with the Specification C271 Minor Concrete Works.
- 4. Except where concrete encasement is ordered by Council, the Contractor shall sand compact backfill around risers to the top of the socket or coupling on the highest branch off the riser, for the full width of trench and for a minimum distance of 500mm upstream and downstream of the riser. Compaction density shall be as for the requirements for the trench pipeline.

### C402.27 MARKING OF JUNCTIONS AND PROPERTY CONNECTION SEWERS

- 1. The Contractor shall clearly mark the position of each riser, junction or end of a property connection sewer on completion of backfilling. The marking shall be made by one (1) of the following methods
  - (a). Where the position of a riser, junction or the end of a property connection sewer is at a substantial boundary fence or structure located on the boundary, a neatly stencilled letter "J" 50mm high shall be painted thereon. An underground identification tape, as specified hereafter, shall finish flush with the existing ground surface as close to the boundary fence or structure as possible.
  - (b). Elsewhere, the Contractor shall drive into the ground, a peg, 75mm x 50mm x **Peg** 600mm long at that position, and left flush with the surface of the surrounding ground. The Contractor shall connect the peg to an underground identification tape as specified hereafter.
  - (c). The Contractor shall tie the identification tape to the junction or end of the property connection sewer and hold the tape in a vertical position during backfilling. The Contractor shall spike the top end of the tape by the junction peg immediately upon completion of backfilling. (WSA 02, 3, 17.11)
- Yellow detectable marking tape shall be laid along the line of all junctions. The tape is to be positioned at either the interface between the bedding material and the backfill material, or 150mm above the top of the service when the backfill material is the same as the bedding material.

### C402.28 TRENCH STOPS

- Where a sewer or rising main is laid on bedding at a grade of 5 per cent to 14 per cent, the Contractor shall construct, as below, trench stops consisting of bags filled with clay, or sand or cement stabilised sand and sealed: (WSA 02, 1, 8.10 & 3, 17.5, SEW-1206, SEW-1207)
- (a) At the socket side of the joint nearest to the position of a stop required in accordance with the formula hereinafter, a recess 100mm deep to suit © The AUS-SPEC Joint Venture date: Jan 2002 Copying for on selling strictly prohibited

Concrete

Encasement

<sup>5.</sup> All property connection sewers and junctions shall have a minimum diameter of 150mm and have a screwed access cap. Property connection sewers shall have **Connection** a maximum length of 10m. **Sewer Caps** 

the width of bag shall be excavated into the bottom of the trench across its full width and into both sidewalls and extend to within 150mm below finished surface level, or such other level as directed by Council.

- (b) The bags shall be placed around and above the pipe, as in (a) above, so as to give close contact with the pipe and to fill the entire space between the excavated recess and the pipe. Bags shall not be placed onto sand bedding.
- 2. The distance between trench stops shall be determined by the following formula: **Spa**

Spacing

 $D = \frac{100}{G}$ , whereby

D = Distance between stops in m,

G = Grade of pipe expressed in percentum.

### C402.29 CONCRETE BULKHEADS

- Where a gravitation sewer or rising main is installed at a grade of 15 per cent to 29 per cent, the Contractor shall construct concrete bulkheads. Where a pipe is installed at a grade of 30 per cent and over, the Contractor shall construct concrete bulkheads integral with concrete encasement. Bulkheads shall be of 20MPa concrete complying with the standard drawing used and C271 – Minor Concrete Works, and 150mm minimum thickness as follows: (WSA 02, 1, 8.10 & 3, 17.6)
  - (a) Where concrete bedding or encasement to pipe is required, the 150mm thick bulkhead shall be cast integral with the concrete bedding or encasement across the width of trench and shall be keyed into both sidewalls a minimum of 150mm. The bulkhead shall extend to 150mm below finished surface level or such other level as directed by Council.
  - (b) Where other bedding, or no bedding, is applicable, the bulkhead shall also be keyed into the bottom of the trench 150mm for the full width of trench.
  - (c) A 75mm nominal diameter drain hole shall be provided in the concrete bulkhead immediately above the top of the encasement bedding or foundation and crushed rock or gravel shall be placed in and at the upstream end of the drain hole to act as a filter. The gravel shall be 10 to 20mm in size within 150mm in all directions upstream and above the invert of the drain hole beyond which another 150mm thick surround of gravel 2 to 10mm in size shall be placed.

Grade 15% to 29% and 30% and over

2. The distance between concrete bulkheads shall be determined by the following **Spacing** formula:

Concrete bulkhead

Concrete encasement (continuous) and concrete bulkhead

$$D = \frac{100}{G}$$
, whereby

L = 80 X Pipe length, m

= 450 m max

if L> 100 m use intermediate trench stops at spacing < 100/G

D = Distance between bulkheads in m

G = Grade of pipe expressed in percentum

### C402.30 THRUST AND ANCHOR BLOCKS FOR RISING MAINS

1. The Contractor shall construct thrust and anchor blocks where shown on the Location design plans to the dimensions depicted therein or as otherwise directed by Council. The blocks shall be provided at valves, flexibly jointed bends, tees, enlargers and reducers or any other point where unbalanced forces resulting from internal pressures will occur. 2. The Contractor shall provide permanent thrust blocks of 20MPa concrete, **Thrust Blocks** complying with C271 - Minor Concrete Works, such that the thrust blocks bear against undisturbed material normal to the direction of thrust resulting from internal pressures over the bearing area not less than that directed by Council. 3. The Contractor shall provide permanent anchor blocks of 20MPa concrete, Anchor Blocks complying with C271 - Minor Concrete Works, of a volume not less than that shown on the design plans. The Contractor shall provide temporary anchorages adequate to restrain the pipe 4. Temporarv when under test. Anchorage The Contractor shall obtain the consent of Council for the type and use of 5. Restrained restrained joints, as an alternative to thrust blocks, in the case of congested Joints service corridors and urgent commissioning. C402.31 **RISING MAIN FITTINGS** 1. The Contractor shall install rising mains, air release valves and inspection pipes Location where shown on the design plans or directed by Council. All rising mains shall be topped with an appropriate identification tape. 2. The Contractor shall provide marking plates bearing the letters "AV" for air Marking Plates valves, "SCOUR" for scour pipes and "SRM" for sewage rising main at changes of direction and at such chainages that the location of the main is marked, at least once each 100 metres, as specified hereinafter. In urban areas, the kerb adjacent to each fitting is to be painted with two (2) coats of non-slip paint, white background with red lettering.

Post Details

- 3. Where, in the opinion of Council, a valve or fitting is at too great a distance from any existing wall, fence or post to which the notice plate could be conveniently fixed, the Contractor shall provide and set in the ground a post with the relevant marking plate fixed at the top of the post, facing the fitting. The distance to the fitting in metres, to an accuracy of 0.1m, shall be permanently marked on the plate with legible numbers a minimum 80mm high. Wooden posts shall not be used.
- 4. The post shall conform to the following requirements:
  - (a) The post shall be of sufficient length to be set firmly in place under saturated ground conditions.
  - (b) When installed, the post shall project 1000mm above the ground, provided that where tall grass or crops are likely to obscure the post, or where directed by Council, its height above the ground shall be increased to 1500mm.
  - (c) The post shall be painted with 2 coats of white enamel for exterior use.

### C402.32 CONCRETE ENCASEMENT

- The Contractor shall encase in concrete pipes in gravity sewers or rising mains, as shown on the design plans, with less than the specified cover above the top of the pipe barrel, or where directed by Council. Concrete shall be 20MPa complying with standard drawings used and C271 – Minor Concrete Works and have the following minimum dimensions:
  - (a) For trenches in other than rock: 150mm minimum under, on both sides and on top of the pipe barrel.
  - (b) For trenches in rock: 100mm minimum under the pipe barrel, 150mm on top of the pipe barrel and for the full width of trench excavated.
- 2. In trenches of other than rock or fissured rock, a contraction joint consisting of a layer of bituminous felt 12mm thick shall be formed in the concrete encasement **Joint** at the face of each socket or at one (1) face of each coupling.
- 3. Reinforcement in concrete encasement shall be as shown on the design plans. *Reinforcement*

### C402.33 WRAPPING OF PIPELINES

- 1. Where shown on the design plans or directed by Council, the Contractor shall enclose a pipeline or a section thereof, in layflat polyethylene sleeving. (WSA 02, 3, 17.10).
- The materials to be used shall be high impact resistance polyethylene sleeving of minimum thickness 0.2mm polyethylene film, approved by Council, and 50mm wide plastic adhesive tape.
- 3. The width of the sleeving when flat shall be in accordance with the pipe **Width** manufacturer's written recommendations for the size and type of the pipeline which is to be encased. Precautions shall be taken so that exposure to direct sunlight does not exceed 48 hours.
- 4. Where necessary to distinguish pipes within close proximity, pipelines shall be identified by colour sleeving, green in colour, or an appropriate identification tape.
- 5. Application of the polyethylene sleeving and plastic adhesive tape shall be in **Application** © The AUS-SPEC Joint Venture date: Jan 2002 Copying for on selling strictly prohibited

accordance with the pipe manufacturer's written instructions or as directed by Council. The Contractor shall take due care not to damage the sleeving during its application or during the backfilling of the trench. Each pipe shall be encased in a length of sleeving overlapped for a minimum of 250mm at each field joint, and the ends of each length of sleeving shall be held in position with at least three (3) circumferential turns of adhesive tape. As the polyethylene sleeve material covering the pipe will be loose, excess material shall be neatly drawn up around the pipe barrel, folded into an overlap on top of the pipe and held in place by means of strips of plastic tape at approximately one (1) metre intervals. Bends, tapers and similar fittings shall be covered by polyethylene sleeving as specified for the pipes. The Contractor shall hand wrap valves, hydrants and irregular shaped fittings and joints using flat polyethylene sheets secured with plastic adhesive tape, or other suitable material, to provide an adequate seal. The flat polyethylene sheets may be obtained by splitting suitable lengths of sleeving.

6. The Contractor shall rectify any damage done to the polyethylene tubing before, **Damage** during or after backfilling of the trench.

### C402.34 CORROSION PROTECTION OF STEEL BOLTS AND NUTS

 The Contractor shall wrap all galvanised steel bolts and nuts, used for installation below ground, of flanges, bolted gland joints, mechanical joints, tapping bands using a tape, approved by Council consisting of synthetic fibre open weave cloth impregnated with saturated hydrocarbons applied in accordance with the manufacturer's recommendations. Bolts and nuts shall be dry, clean and free from rust immediately before wrapping.

### C402.35 CAST-IN-SITU MAINTENANCE HOLES

- For all maintenance holes concrete work, the Contractor shall comply with C271

   Minor Concrete Works in relation to the supply and placement of concrete and steel reinforcement, formwork, tolerances, construction joints, curing and protection except as specified below. (WSA 02, 3, 18.5).
- Cement used in all concrete shall be Type SR to AS 3972. The Contractor may
  use fly ash additive to a maximum 20 per cent. Cement used shall be no older
  than three (3) months since manufacture.
- 3. The minimum cement content shall be 360 kg/m<sup>3</sup> of concrete and the **Minimum** water/cement ratio of the mix shall not be greater than 0.50 by mass. **Cement** Content

### C402.36 COVERS AND FRAMES

- 1. Covers and frames shall not be warped or twisted. Surfaces shall be finished such that there are no abrupt irregularities and gradual irregularities shall not exceed 3mm. Unformed surfaces shall be finished to produce a surface that is dense, uniform and free from blemishes. Exposed edges shall have a minimum 4mm radius. (WSA 02, 3, 18.1 & 18.9 & 19.3). Covers and frames shall not be delivered to the site before satisfactory documentary evidence has been submitted to Council that quality tests have been carried out. This action constitutes a HOLD POINT. Council's approval to the quality test documentation is required prior to the release of the hold point.
- 2. Tolerances for the dimensions on the COVER shall be 3mm + NIL. Cover Tolerance
- 3. Tolerances for the dimensions on the FRAME shall be -3mm +3mm.

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Standard

HP

ΗP

Watertight

**Components** 

Component

Manufacturers'

**Procedures** 

Assembly

- 4. Maintenance hole covers shall be seated on a layer of bitumen-impregnated Cover Seating fibreboard, having a cross-section of 25 x 25mm. Alternatively another seating material of a cross-section and composition approved by Council may be used.
- 5. Maintenance hole covers shall be finished flush with the surface in roadways, **Cover Levels** footpaths and paved surfaces of any type. Elsewhere, covers shall be finished 25mm above the surface of the ground where not shown otherwise on the design plans, or such other level as directed by Council, in a manner designed to avoid as far as possible, the entry of surface water.
- In locations where shown on the design plans or directed by Council, the Ductile Iron 6. Contractor shall install a ductile iron cover and frame instead of the standard Cover concrete maintenance hole cover. Where shown on the design plans, the Contractor shall install bolt down frames and covers in areas subjected to 1 in 100 year flooding. Ductile iron covers and frames shall be manufactured in accordance with AS 3996, and shall be installed as necessary, in accordance with the manufacturer's written requirements.

#### C402.37 **STEP IRONS**

Step irons are not normally required, but if specified on the design plans they 1. Fixing shall be plastic encapsulated steel. The Contractor shall fix step irons in the concrete, ensuring step hold, alignment and spacing is positioned for safe access. (WSA 02, 3, 18.1, SEW-1307).

#### PREFORMED MAINTENANCE HOLE AND MAINTENANCE SHAFT C402.38 SYSTEMS

- 1. If approved by Council, preformed systems, complying with the design plans, if Approval any, otherwise complying with AS 3518, AS 3571 or AS 4198 may be used in lieu of cast in-situ systems. (WSA 02, 3, 18.4). Preformed system components shall not be delivered to the site before satisfactory documentary evidence has been submitted to Council that quality tests have been carried out. This action constitutes a HOLD POINT. Council's approval to the quality test documentation is required prior to the release of the hold point.
- 2. The Contractor shall supply components that make a watertight system and have a satisfactory surface finish.
- 3. Generally, preformed maintenance holes shall be made up in accordance with the design plans, with components consisting of a base section, shaft sections of section lengths such as to minimise the number of joints required, a cone section, cover and frame. Make-up Rings may be used between cone sections and frames to make up height differentials. The wall thickness of any reinforced component below the frame shall not be less than 84mm. The vertical distance from the top of the surround and the first step is to be in the range of 600mm to 900mm.
- 4. Generally, preformed maintenance shafts shall be made up in accordance with Maintenance the design plans, with components consisting of a base section, shaft sections of Shafts section lengths such as to minimise the number of joints required, cover and frame. (WSA 02, SEW-1314)
- 5. The installation of all preformed components shall be in accordance with the manufacturers' recommended procedures and requirements.
- 6. Backfill for all preformed maintenance holes and maintenance shafts shall be Backfill placed and compacted evenly around the maintenance hole to a level 300mm above the top of the highest incoming pipe and for the full width of the excavation. If necessary, the Contractor shall import and compact non-cohesive

granular material.

### PIPELINE TESTING AND RESTORATION

### C402.39 GENERAL

- 1. The Contractor shall subject all sewers and maintenance holes to an initial test as soon as practicable after construction and before backfilling is commenced. An acceptance test shall be carried out before the issue of the Certificate of Practical Completion and not earlier than one (1) month after completion of construction of all sewers and maintenance holes in a section. Sewers or maintenance holes failing any test, shall be repaired and the test repeated. The process of testing, repair of defects and retesting shall continue until a satisfactory test is obtained. (WSA 02, 3, 22).
- 2. All lines shall be clear and free from soil, slurry, liquids and other foreign **Cleaning** substances at the time of initial and acceptance testing.
- 3. Where a vacuum system has been specified, the Contractor shall test the system in accordance with the testing schedule as shown on the design plans.

### C402.40 INITIAL TEST OF GRAVITATION SEWERS

- The Contractor shall make the initial testing of gravitation sewers with compressed air. Before the initial test is performed, all pipelaying on the section shall be completed, and backfill shall be compacted to the level of the centre of the pipe barrel and Council notified. This action constitutes a WITNESS POINT. Council shall advise at the time of notification by the Contractor whether the option to inspect the initial testing is required.
- 2. The initial test may be carried out before risers and/or property connection sewers are constructed so that the main line can be backfilled. However, the Contractor shall carry out an initial test on the risers and property connection sewers as soon as they are completed.
- 3. Where Council approves the construction of pipelines in other than full lengths **O** between maintenance holes, each length of pipeline shall be tested before backfilling together with the downstream portion of the maintenance hole length under construction.
- 4. The Contractor shall rectify any fault detected and obtain a satisfactory test **Re** before the remainder of backfill is placed.
- 5. The Contractor shall undertake ovality testing as follows:
  - (a) All sewers to DN 300 shall be tested to determine any excessive ovality using a proving tool approved by Council. Ovality testing shall be undertaken after all earthworks on the subdivision are complete and no sooner than 28 days after backfill of trenches has been completed. Sewer pipes having excessive ovality shall be replaced and the line retested.
  - (b) The proving tool shall be rigid and non-adjustable having an effective length of not less than its nominal diameter. The minimum diameter at any point along the length shall be:

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Initial Test Before Backfill

Compressed Air

Vacuum

System

WP

Risers and Property Connection Sewers

Other Than Full Lengths

Rectification

**Ovality Testing** 

NOMINAL SIZE (DN)	MINIMUM PROVER DIAMETER (mm)	
	UPVC PIPE	
100	99.7	
150	142.6	
225	222.9	
300	280.8	

- (c) The proving tool shall be fabricated from steel and have pulling rings at each end. The prover shall be marked to indicate the nominal pipe size and the prover outside diameter.
- (d) Maximum Allowable Deflection = 3% of Mean Outside Diameter.
- (e) The testing shall require a "prover" to be pulled through each section of the pipeline by hand winching to demonstrate that the maximum allowable deflection is not exceeded.

### C402.41 INITIAL TEST OF MAINTENANCE HOLES

1. After the maintenance holes have been constructed (including benching, fitting of the converter slab, surround frame and cover) and all backfill operations are complete, they shall be tested as a minimum number as hereafter specified in Table C402.5. (WSA 02, 3, 22.4.4)

Number of Sewer Maintenance Holes in Subdivision Works	Percentage Tested Initially (see Item 2)
≤ 5	100%
6 to 10	50%
11 to 20	33%
> 20	25%

### Table C402.5

- 2. Where projects contain both precast and in-situ concrete Maintenance Holes, each type shall be viewed as separate populations, with the above criteria applying to each population separately within the project. If any of the sample holes fail the initial test, then all Maintenance Holes within the project shall be tested.
- 3. The test shall be made by plugging all pipe openings in the walls, the vacuum **Method** test head shall be placed in the top of the Maintenance Hole and the seal inflated. Draw a vacuum of 33.5Kpa on the Maintenance Hole then close the valve on the vacuum line and turn the pump off.
- 4. The Maintenance Hole shall have passed the vacuum test if the time taken for **Duration** the reading to drop to 30 KPa meets or exceeds the time specified in Table C402.6

Maintenance Hole Depth (mm)	Time in Seconds
<2400	17
3000	21
4000	28
5000	35
6000	42

### Table C402.6

5. Alternatively, the Maintenance Holes may be tested by alternative methods, in which case the Contractor shall provide details of the alternative method *Alternative* proposed, for approval by Council, prior to its use.

## C402.42 ACCEPTANCE TEST OF GRAVITATION SEWERS AND MAINTENANCE HOLES

to blow off at 35kPa  $\pm$  4kPa and a gate valve to the pipeline to be tested.

1.	The Contractor shall make the acceptance test on all components in the section of the sewer in the same manner as the initial test. The submission, to Council, of satisfactory test results constitutes a <b>HOLD POINT</b> . The approval of Council	As for Initial Test
	is required prior to the release of the hold point.	HP
2.	Council may permit hydrostatic testing as an alternative to compressed air testing for acceptance of gravitation pipelines.	Alternative
3.	Council may reject any pipeline or maintenance hole in which there is visible or detectable leakage.	Rejection
C402.4	43 TESTING GRAVITY MAINS WITH COMPRESSED AIR	
1.	The Contractor shall supply and keep all necessary equipment in a condition acceptable to Council.	Equipment
2.	The Contractor shall test pressure gauges prior to use by static water column.	Pressure Gauges
3.	Compressed air shall be supplied by a compressor of the rotary vane type capable of supplying at least 1 m <sup>3</sup> /minute at 35kPa. The air shall be fed through a pressure-reducing valve capable of reducing pressure from that supplied to $35kPa \pm 4kPa$ . The air shall then pass through an airtight line fitted with a pressure gauge reading from 0 to 50kPa, a pressure relief valve that shall be set	Compressed Air

- 4. The method of setting up and carrying out the test shall be as follows: (WSA 02, *Method* 3, 22.4)
  - (a) Insert a blank plug at one end and a disc with air-hose connection at the other end of the line. Care shall be taken to ensure that the force due to pressure on the disc is not taken by pipe joints, but is taken by struts bearing on the disc or on the end pipe in the line. Test lengths shall be limited to single pipe runs between MHs and/or MSs.
  - (b) Couple test equipment to line under test and compressor or airline.
  - (c) Slowly increase the air pressure in the line from 0 to 35kPa (over one (1) minute approximately).
  - (d) Hold air pressure at 35kPa for three (3) minutes for stabilising temperature.
  - (e) Close gate valve to shut off air supply to test equipment.
  - (f) Measure the time it takes for the pressure to drop from 35kPa to 28kPa. If this time is less than that permitted or if the line cannot be pressurised to 35kPa, then the test is unsatisfactory and the pipeline shall be checked for leaks.
  - (g) To check pipelines for leaks:
    - I. Open the gate valve from the air supply sufficiently to maintain a pressure of 14 to 23kPa in the pipeline.
    - II. Move along the pipeline coating it with detergent solution. Bubbles will indicate a point of leakage. Special attention should be paid to joints, discs and horns of junctions.
  - (h) If leaks are detected, they shall be repaired to the satisfaction of Council.
  - (i) Re-test as above until the time taken for the pressure to drop is greater than that shown below.

### C402.44 ALLOWABLE PRESSURE DROP TIMES

1. The time taken for the pressure to drop from 35 kPa to 28 kPa shall be greater *Time* than:

225mm pipe or smaller – 3 minutes 300mm pipe – 6 minutes 375mm pipe – 8 minutes 400mm pipe – 11 minutes 525mm pipe – 14 minutes 600mm pipe – 17 minutes

 Pressure drop times which are less than these may indicate leakage or excessive air permeability through unsaturated pipe walls with some materials. Vitrified clay pipes, in particular, suffer from excessive air permeability under dry summer conditions. When this occurs, pipes shall be thoroughly saturated with water before testing or a hydrostatic test applied.

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3. In any case, where the allowable pressure drop time cannot be attained and **Hydrostatic** there are no visible leaks, the Contractor shall apply a hydrostatic test. Test C402.45 HYDROSTATIC TESTING OF GRAVITY MAINS 1. The Contractor shall carry out the hydrostatic test by connecting to the pipeline Pipe Connection or section thereof under test, a pipe or hose terminating in a 150mm diameter container not less than 100mm deep. All other open ends of the pipeline shall be plugged. 2. The pipeline under test, and the pipe or hose with container, shall be filled with Water water until the free surface is level with the top of the container, when that container is suspended in accordance with the requirements set out below. 3. The test container shall be suspended at a level such that the test head applied **Test Container** to the pipeline is as follows: For initial test when no property connection sewers or risers are (a) (i) constructed – a minimum head of 2 metres above the pipe invert at the upstream end of the line under test, or (ii) For initial test where property connection sewers and/or risers are constructed - a minimum head of 2 metres above the highest invert in the line under test, including its risers and property connection sewers. (b) For acceptance test, a minimum head of 2 metres above the highest invert in the line under test, including its risers and property connection sewers, or above the free standing level of ground-water in the vicinity whichever is the higher. (c) Such other lesser head as Council, at Council's discretion, may direct. The Contractor shall determine, at the Contractor's expense, the free standing 4. Ground-Water level of groundwater, by a method acceptable to Council. After allowing an interval for absorption, to be determined by Council, any fall of 5. Extra Water the free water surface shall be made good by adding extra water to the container. The Contractor shall measure the fall in water level during ten (10) minutes thereafter. 6. The pipeline will be regarded as satisfactory if there are no visible leaks, and if Results the fall in water level is not more than 25mm for each standard test length of the pipeline under test including property connection sewers and/or risers. A standard test length in metres is defined as 1370m divided by the effective 7. Test Length diameter of the pipeline in millimetres. Where the pipeline under test is all of the same size, the effective diameter shall be the nominal size of that pipeline. Where the pipeline under test has property connection sewers and/or risers of smaller nominal size than the main sewer line, then the effective diameter shall be calculated as the product of the length and the nominal size of the larger pipe added to the product of the length and the nominal size of the smaller pipe; this sum shall be divided by the total length of pipeline under test; the result shall be the effective diameter.

### C402.46 VISUAL INSPECTION AND MEASUREMENT OF INFILTRATION

1. Whenever, in the case of acceptance testing, the pipeline is subjected to a significant head of groundwater (i.e. 1500mm or more above the soffit of the Groundwater

Contractor's

Cost

sewer main provided that groundwater is at least 150mm above any property connection sewer included in the test), the tests previously prescribed may be dispensed with in favour of visual inspection and measurement of infiltration.

- 2. In such circumstances, the Contractor shall propose full details of the method by **Method** which the infiltration is to be measured.
- 3. If Council, at it's discretion, approves of an inspection and infiltration test being performed for the purposes of acceptance, Council shall determine, the duration over which infiltration is to be measured. The rate of infiltration shall not exceed that determined by the following formula:-

Q.I. = 0.65 
$$(L_1d_1h_1 + L_2d_2h_2 + \dots L_nd_nh_n) + H_a$$

Where:

- Q.I. = rate of infiltration in litres/hour
- L = length of pipe in metres
- d = nominal size of pipe in metres
- h = average head of groundwater over the invert level of the pipe in the section under test
- H<sub>a</sub> = head of groundwater above the invert level of the outlet pipe of the maintenance hole when the maintenance hole is included in the infiltration test.
- 4. The Contractor shall determine the head of groundwater, at the Contractor's expense, by a method approved by Council.

### C402.47 TESTING OF RISING MAINS

1.		ontractor shall pressure test rising mains to detect leakage and defects in peline including joints, thrust and anchor blocks. The submission, to	HP
	Counci	I, of satisfactory test results constitutes a <b>HOLD POINT</b> . The approval of I is required prior to the release of the hold point.	
2.		es shall be tested in sections approved by Council as soon as practicable ach section has been laid, jointed and backfilled, provided that:	Timing
	(a)	If so specified or if the Contractor so desires, some or all of the pipe joints shall be left uncovered until the whole of the section has been successfully pressure tested to the satisfaction of Council; and	
	(b)	The pressure testing shall not be commenced earlier than seven (7) days after the last concrete thrust or anchor block in the section has been cast.	
3.		e purpose of this clause, a section shall be defined as a length of pipeline can be effectively isolated for testing, eg by means of main stop valves.	Section Definition
4.		re testing shall not be carried out during wet weather unless otherwise ed by Council.	Wet Weather
5.		pressure testing, all field joints which have not been backfilled shall be dry and accessible.	Field Joints
6.	once, t	the pressure testing of a pipeline, each stop valve shall sustain at least he full test pressure on one (1) side of the valve in closed position with no re on the other side for at least 15 minutes.	Stop Valves
7.	of Cour of air f	testing a pipeline section, the Contractor shall clean it to the satisfaction ncil and fill it slowly with water, taking care that all air is expelled. Purging from rising mains shall be promoted by opening air valves. In order to e conditions as stable as possible for testing by allowing for absorption,	Filling with Water

movement of the pipeline and escape of entrapped air, the section shall be kept full of water for a period of not less than 24 hours prior to the commencement of the pressure testing.

- 8. The hydrostatic test pressure which shall be applied to each section of the **Test Pressure** pipeline shall be equivalent to the pressure rating of the pipe specified.
- 9. The Contractor shall maintain the specified test pressure for as long as required by Council, while the Contractor examines the whole section. In any case, the specified test pressure shall be maintained for not less than 8 hours. For the purpose of determining the actual leakage losses, the Contractor shall carefully measure and record the quantity of water added in order to maintain the pressure during the period of testing. A Council approved and supplied water meter shall be fitted to measure leakage from the rising main.
- 10. The pressure testing of a section shall be considered to be satisfactory if: **Results** 
  - (a) There is no failure of any thrust block, anchor block, pipe, fitting, valve, joint or any other pipeline component;
  - (b) There is no visible leakage; and
  - (c) The measured leakage rate does not exceed the permissible leakage rate as determined by the following formula:

$$Q_1 = (0.000532 + C_p) D.L. (H)^{0.5}$$

where:

Q<sub>1</sub> = permissible leakage rate (litres per hour)

- C = a coefficient as specified hereunder for the particular pipe material and type of joint
- D = nominal diameter of pipe (mm)
- L = length of section tested (km)
- H = average test head (m)
- $L_p =$  average pipe length <u>L</u> (m)

where "n" is the total number of pipes and fittings in the section tested.

n

(d) the measured leakage rate does not exceed that rate calculated by the simplified formula for the type of pipe tabulated hereunder, in which event determination of the permissible leakage rate on the basis of the formula specified in I above shall not be necessary. The simplified formulae are based on the coefficient "C" and average pipe lengths contained in that tabulation.

Pipe	Simplified	Coefficient "C"	Average Pipe
Type	Formula		Length (m)
D.I.	$Q_1 = 0.0105 \text{ D.L. (H)}^{0.5}$	0.0548	5.5
PVC	$Q_1 = 0.01 \text{ D.L. (H)}^{0.5}$	0.0568	6.0

Rectification

detected during the pressure testing of the pipeline or during the Defects Liability Period shall be rectified by the Contractor at the Contractor's expense. 12. Alternatively, the rising main may be tested by the use of compressed air. In this Alternative case, the Contractor shall provide details of the alternative method proposed, for Tests approval by Council, prior to its use. C402.48 **BACKFILL AND COMPACTION** 1. After laying and jointing of a pipeline has been completed the Contractor shall Notification present the laid and jointed pipes for inspection by Council prior to commencement of trench backfilling (WSA 02, 4, 21). This action constitutes a WP WITNESS POINT. 2. Backfill shall not be placed until Council has given approval. Approval 3. Material for the side support and overlay of the pipe shall be as for pipe bedding Side Support specified in Clause C402.23. The material shall be compacted in layers of not and Overlav more than 150mm to 95 per cent of the standard maximum dry density of the material used when determined in accordance with AS 1289.5.7.1. 4. The Contractor shall backfill the remainder of the excavation and compact the Remainder of backfill in layers of not more than 150mm thick as follows: (WSA 02, 3, 21). Trench Where the trench is within a roadway, proposed roadway, or footpath (a) area, the remainder of the trench shall be: Backfilled with a non-cohesive granular material, with a grading Backfill to (i) falling generally within the limits shown in Table C402.3, and Subarade compacted to Density Index of 70 when determined in Level With accordance with AS 1289.5.4.1 for cohesionless materials Non-Cohesive Granular 1. Below 0.5m of the road surface 2. In the road reserve, but excluding the road pavement Backfilled with excavated material, and compacted to 100 per (ii) Backfill to cent of the standard maximum dry density of the material when Subgrade determined in accordance with AS 1289.5.7.1, to within 0.5m of Level with the road surface, but excluding the pavement layers. Excavated Material (iii) Backfilled with road base and sub-base material as per existing Backfill of or proposed pavement layers and compacted to 100 per cent of Pavement the standard maximum dry density of the material when Layers determined in accordance with AS 1289.5.7.1 (b) Elsewhere, unless stated otherwise, the remainder of the trench shall be backfilled with ordinary excavated backfill material. Where suitable material is not available, granular material may be used for the full depth of backfilling. The material shall be compacted to a density Index of 70 when determined in accordance with AS 1289.5.4.1 for cohesionless materials or 98 per cent of the standard maximum dry density of the material when determined in accordance with AS 1289.5.7.1 for cohesive materials. 5. The Contractor shall carry out backfilling and compaction without damaging the Care pipe or its external coating or wrapping or producing any movement of the pipe.

Any failure, defect, visible leakage and/or excessive leakage rate, which is

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11.

- 6. The Contractor shall carry out compaction tests 75mm to 100mm below the level being tested. (WSA 02, 3, 22.3)
- 7. The Contractor may compact backfill by trench flooding only where:
  - (a) The ground and backfill material is cohesionless sand.
  - Water for flooding has been sourced at the site. (b)
  - The process will not create mud which would be moved off site by (c) vehicles or construction plant.
  - Additives are not used. (d)

#### **RESTORATION OF SURFACES** C402.49

- 1. The Contractor shall clean pavements, lawns and other improved areas and Original leave them in the same order as they were at the commencement of the Works. The Contractor shall restore any fencing removed during construction and shall restore lawns with turf cut and set aside from the original surface and with imported turf from a source approved by Council. (WSA 02, 3, 25).
- 2. The Contractor shall maintain all restored surfaces in the condition to which they are restored until the expiry of the Defects Liability Period applicable to those surfaces, notwithstanding that any deterioration of the restored surfaces, and the need for their maintenance may or may not be due to defects which become apparent or arise from events which occur during the Defects Liability Period. The Contractor shall maintain pavements with crushed igneous rock, gravel or other suitable material allowing for consolidation and shall then restore them to a condition equivalent to that of the original pavement.
- 3. Immediately the backfilling of a trench excavated through a pavement has been completed, the Contractor shall temporarily restore the pavement. Where the trench crosses bitumen or concrete pavement, the surface is to be protected from deterioration. A pre-mixed asphaltic material may be used for such temporary restoration. The Contractor shall maintain the temporary restoration until final restoration is carried out. Final restoration of the pavement shall be carried out to restore the pavement and its sub-base to no less than the original condition. Final restoration may include, if required by Council, the removal of temporary restoration.
- 4. In other than roadways, the Contractor shall place the backfill sufficiently high to compensate for expected settlement and further backfilling shall be carried out or the original backfill trimmed at the end of the Defects Liability Period in order that the surface of the completed trench may then conform with the adjacent surface. Surplus material shall be removed and disposed of to areas arranged by the Contractor. Where dry weather conditions have persisted after the original backfilling, including during the Defects Liability Period, the Contractor shall take all necessary steps to consolidate the trench before removing surplus materials from the site.
- 5. In locations where, in the opinion of Council, surplus material left in the vicinity of the trench would not be objectionable, the surplus material may be disposed by spreading neatly in the vicinity of the trench to the satisfaction of Council in such a way as to avoid future erosion of the backfill and adjacent ground surfaces. The Contractor shall maintain the backfill and adjacent ground until the expiry of the Defects Liability Period.
- 6. Where, within public or private property, the reasonable convenience of persons Settlement will require such, Council may order the Contractor to level trenches at the time © The AUS-SPEC Joint Venture date: Jan 2002 Copying for on selling strictly prohibited

Condition

Compaction

Tests

Flood Compaction

### Maintenance

Temporary Pavement Restoration

Backfill

Disposal of Surplus Material

Northern Rivers – Local Government

of backfilling. The Contractor shall make good any subsequent settlement, as required by placing additional fill.

- 7. The Contractor shall immediately restore any damaged or disturbed private **Restoration** property and services.
- 8. Should the Contractor elect to tunnel under paving, kerb and gutter or other **Tunnelling** improved surfaces in lieu of trenching, backfilling shall be so carried out as to restore full support to those surfaces. The Contractor shall remain responsible for the repair of the improved surfaces, if subsequently damaged due to subsidence of the backfill, until the end of the Defects Liability Period.
- 9. The Contractor shall provide notice to affected property owners of any pending *Property Owner Advice*

### **PUMP STATIONS**

### C402.50 PUMPS

- 1. Pumps shall be submersible sewage pumps. Council's preferred submersible *Materials* pumps are: Flygt, Mono, Forrers and Grundfos.
- 2. Pump construction materials for centrifugal end suction pumps shall comply with the following:

DESCRIPTION	MATERIAL
PUMP	
Casing and suction bend	Cast iron AS 1830 Gr T200
Wear rings	Cast iron AS 1830 Gr T200
Impeller	316 Stainless steel/AS 1449
Impeller nut	Gunmetal AS 1565-905C
Shaft	316 Stainless steel/AS 2837
Shaft sleeve	Phosphor bronze AS 1565-9060/316
Neck bush, lantern ring	Phosphor bronze AS 1565-9060
Gland	Cast Iron AS1830 Gr T200
Gland studs	316 Stainless steel/AS 2837
Gland nuts	316 Stainless steel/AS 2837
Fixing nuts and bolts handhole	316 Stainless steel/AS 2837
Covers	316 Stainless steel/AS 1449
Fitted bolts and nuts, casing and dowels	316 Stainless steel/AS 2837
Forcing screws	316 Stainless steel/AS 2837
Water thrower and drip tray	316 Stainless steel/AS 1449
Pump set base plate	Cast iron AS 1830 Gr T2000/Fabricated steel
MOTOR	
Motor frame and end shield	Cast iron/Mild steel
Motor terminal box	Cast iron/Mild steel

DESCRIPTION	MATERIAL
Motor fan cover	Mild steel
Motor fan	Metal
HOLDING DOWN BOLTS	316 Stainless steel/AS 2837
MECHANICAL SEALS	
Seal faces	Tungsten carbide or equal
Springs	Nickel chrome steel
Secondary seal	Fluoro carbon or nitrile rubber

3. The Contractor shall provide a written warranty from the Manufacturer of the HP equipment. This action constitutes a HOLD POINT. Council's approval of the warranty is required prior to the release of the hold point. 4. The Manufacturer's warranty shall require the Manufacturer to accept liability for Manufacturer's any defect in materials or workmanship which becomes apparent at any time Liability within two (2) years after the date of delivery of any piece of equipment used in the subdivision works. 5. All nuts and bolts shall be manufactured in accordance with AS/NZS 1111 and Nuts and Bolts AS/NZS 1112, 150 metric series and fitted with washers beneath bolts heads and nuts. All bolts, nuts and washers shall be stainless steel to AS 1449 and (a) AS 2837, minimum grade 316. All bolts, nuts and washers are to be of the same grade and supplied passivated. (b) All threads are to be rolled. All bolt heads and nuts shall be hexagonal. (C) (d) All bolts, studs, set screws and nuts for bolting flanges and other pressure containing purposes shall conform to AS 2528. All nuts and bolts subjected to vibration shall be fitted with lock washers (e) or lock nuts. (f) All concrete anchor bolts, nuts, locking nuts and large series washers required for the bolting down of pump set discharge bends shall be provided. These anchor bolts shall be as recommended by the equipment designer with a minimum diameter of 16mm. Concrete anchor bolts shall be chemical masonry anchor type, set to (g) their full depth, suitable for the required duty. Bolts on all flanges will protrude no more than 10mm past the nut when Bolts on 6. tightened. Flanges The Contractor shall apply sufficient anti-seize/anti-galling material to the threads 7. Anti-Galling, of all stainless steel fasteners. The material shall be Polytetrafluroethylene Anti-Seize (PTFE), either tape to AS 1272, dipped or sprayed, or molybdenum disulphide.

## C402.51 PREFORMED PUMP STATIONS AND PACKAGE PUMP STATIONS

1. Preformed components or systems, complying with the design plans, if any, Alternate Wet otherwise complying with AS 3518, AS 3571 or AS 4198 may be used in lieu of Well in-situ construction provided: Preformed concrete wall units are to be manufactured to AS 4058 except (a) as modified as for the requirements for precast maintenance hole units. Joints shall be internal flush (b) (c) The Contractor shall supply components that make a watertight system Component and have a satisfactory surface finish. Quality 2. Package pump stations may be supplied and installed provided: (a) All components comply with the requirements of this Specification (b) The units are at least equivalent to the requirements of this Specification Package Units and the design plans. ELECTRICAL COMPLIANCE C402.52 The Works shall be in accordance with the Electrical Services Minimum Standards 1. Requirements contained in MEW E101 except where this Specification or the design plans indicate otherwise. The technical requirements detailed on the design plans shall take precedence over the requirements of this Specification should clauses be in disagreement. MEW E101 covers the general requirements for materials, workmanship, and **DPWS** 2. methods of installation as follows: Requirements (a) General (b) Reticulation and wiring Switchboards and Associated Equipment (C) (d) Accessories Luminaries – Supply and Installation (e) (f) **Electric Motors** Painting, Colour Coding and Labelling (g) 3. Except where MEW E101 requires a higher standard, Works shall be carried out Compliance in accordance with AS 3000, the Service Rules of the Supply Authority and all relevant Statutory Authorities. 4. The Contractor shall supply proof of compliance with a standard or specified test. Proof of Such proof shall comprise a test certificate from an approved independent testing Compliance authority. 5. The Contractor shall submit all designs and material, to each Authority having Approval jurisdiction for approval. The Contractor shall arrange for each Authority having jurisdiction to inspect the Works. Council shall be advised a minimum of 7 WP working days in advance of the date of any inspection by an Authority. This action constitutes a WITNESS POINT. Council shall advise at the time of

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notification by the Contractor whether the option to attend the inspections is to be

exercised.

## C402.53 SWITCHGEAR AND CONTROL GEAR ASSEMBLY (SCA), CONTROLS

1.	The Contractor shall supply and install the SCA designed and assembled by a manufacturer approved by Council.	Approved Manufacturer
2.	The SCA shall be of outdoor, stationary, free standing, metal-enclosed, cubicle type series with a minimum degree of protection of IP56D as specified in AS 1939.	Туре
3.	All equipment shall be securely mounted on suitable mounting panels and comprise individual compartments. A steel galvanised channel base shall be provided.	Construction
4.	The Contractor shall provide an effective barrier to prevent gases from the wet well entering the SCA.	Barrier to Gases
5.	Starter contactors shall have appropriate ratings for the proposed pumps to AC3.	Starter Contactors
6.	All necessary terminals with terminal and cable numbers shall be supplied and installed in accordance with the design plans.	Terminals
7.	The Contractor shall liaise with the electricity supply authority to supply a lock barrel for the metering equipment, at the Contractor's expense. Council shall supply standard lock barrels for use on the SCA at no cost to the Contractor.	Lock Barrels
8.	The electrical characteristics of the SCA shall be:	Characteristic
	Main Circuit: 415/240 V, 50 Hz, 3-phase, 4-wire. Motor Control Circuit: 240 V, 50 Hz. Common Control Circuit: 240 & 24 V, A.C. Prospective short-circuit current: 14kA for 1 second. Peak Factor: 2.2 Power Factor Correction (Determined in consultation with Council) Earthing (M.E.N. system)	S
9.	All cables shall enter the SCA from below.	Cable Entry
10.	The Contractor shall supply data from the switchgear supplier confirming Type "2" co-ordination between contactors, motor protection relays and corresponding circuit breakers, to Council.	Switchgear Data
11.	The "AUTO" mode shall be capable of being overridden by turning the starter selector switch to the "ON" position. Manual operation would normally be used in the event of failure of the telemetry system or for function testing. A warning label (R/W/R) advising selector switches to be left in the "AUTO" mode shall be fitted to common control cover.	Operation
12.	The Contractor shall carry out of factory tests in the presence of Council's Representative and in accordance with Schedule MEW E101 and the results shall comprise all routine Tests specified in AS 3439. Council shall be given seven (7) days notice of the proposed date of such tests.	Factory Tests
13.	Functional tests referred to in Schedule MEW E101 shall include electrical function tests as defined in AS 3439.	Functional Tests

14.	The Contractor shall pack the equipment for transport after satisfactory final factory inspection and tests, and after approval has been given by Council. The Contractor shall ensure that any relays, programmable logic controllers, and fittings likely to be adversely affected during delivery shall be adequately protected or shall be removed and packed separately in protected containers. Where equipment has been removed, cover plates shall be provided.	Packing
15.	The Contractor shall be responsible for any damage that may occur during transit and unloading at site.	Damage
16.	The Contractor shall ensure that spare parts, tools etc, are packed separately from the main plant and shall be marked "Spare Parts", "Tools" etc, as applicable.	Tools
17.	The Contractor shall supply spare parts in accordance with the schedule supplied by Council.	Spare Parts
18.	Automatic control of the pump station pumping equipment shall be by way of analogue signals.	Automatic Control
19.	The following wet well levels shall be used in the automatic control of the pump operation system:	Levels
	(a) Bottom Water Level (BWL)	
	(b) Top Water Level (TWL)	
	(c) Maximum Top Water Level (MTWL)	
	(d) Flood Alarm Level (FAL)	
20.	In the event of a rise in water level to Maximum Top Water Level. The duty pump will continue to operate and the standby pump will cut in parallel.	Pump Operation
21.	The Contractor shall supply and install control equipment as applicable.	Pump Control
C402.5	64 ELECTRICAL INSTALLATION	
1.	The Contractor shall liaise with the Supply Authority for the electricity supply to the pump station site.	Liaison
2.	The Contractor shall be responsible for all facilities required by the Supply Authority for revenue metering equipment and the payment of all associated connection, inspection fees and capacity charges.	
3.	The Contractor shall supply and install all cabling including consumer mains, motor, control and flow meter cables, conduits and electrical pits.	Cabling
4.	The Contractor shall install all wiring in HD-PVC underground conduits laid in accordance with the Supply Authority's requirements, with a minimum 500mm below the finished ground level in non-trafficable areas and 600mm below the finished ground level in trafficable areas. The trench and backfill material shall be free of rocks and other foreign matter likely to damage the conduits.	Conduits
5.	The Contractor shall run electrical marker tape 150mm below the finished ground level directly above the conduits for the entire length of the conduits. Marker tape shall be orange in colour, 150mm wide and stamped with the words "DANGER – ELECTRIC CABLES BELOW" or similar.	Marker Tape
6.	The Contractor shall route all underground cabling with the approval of Council.	Route
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Brass marking plates shall be positioned on any concrete surround clearly showing the direction of the incoming consumer mains. Wording and markings HP shall read "Danger - Electrical Cables Below". This action constitutes a HOLD POINT. Council's approval of the route of all underground cabling is required prior to the release of the hold point. 7. The Contractor shall determine the Points of Attachment on site and the Point of Contractor shall supply and install any consumer's connection poles for the Attachment consumer mains required by the Supply Authority. The consumer mains shall be generally run underground and commence at the 8. Consumer Point of Attachment on a steel consumers pole (if applicable), installed near the Mains property boundary and run in conduit to the switchboard. 9. The minimum size of the consumers mains shall be sized to satisfy the following Size requirements: (a) Current carrying capacity to suit the maximum demand with an excess current carrying capacity of 30 per cent minimum. (b) Be sized for a voltage drop less than 1.5 per cent to the maximum demand as calculated. Be single core PVC/PVC cables. XLPE insulated cable may also be (c) used. (d) Comply with the requirements of the Supply Authority. (e) Pole termination method shall be as shown on the design plans. (f) AS 3000 and AS 3008 10. In addition to the requirements of the Supply Authority and MEW E101, the Earthing Contractor shall run the main earthing conductor in conduit to the main earthing Conductor electrode. The main earthing connection shall be contained in an earthing electrode connection box similar to ALM type ERB-1 up to 50mm<sup>2</sup> cable and a Type 4 pit for larger cable. 11. The Contractor shall provide a separate earthing conductor and electrode for the Surae surge diverters. Each electrode shall be bonded and suitably labelled with an **Diverters** engraved brass label. 12. The Contractor shall bond the pump station metallic pipework to the main earth. Pipework The Contractor shall install metering facilities within the SCA. The metering 13. Meters facilities and panel shall be Energy Authority approved and suitable for the installation of the metering equipment required by the Supply Authority. 14. The Contractor shall supply and install the following metering equipment: Metering Equipment Plug-in meter bases or all electricity meters (tariffs) supplied by the (a) Supply Authority, as may be required by the Supply Authority. Service potential fuses. (b) (c) Current transformers metering equipment (if required). All necessary wiring and other accessories as required by the Supply (d) Authority. (e) Key locking facilities for Supply Authority access.

connect the on-flow switch and pump motor cables to the appropriate terminals. Cables shall not be jointed.

16. The Contractor shall seal, at the completion of commissioning tests, all conduits **Sealing** into the outdoor SCA with a non-setting sealing compound to prevent the ingress of vermin.

### C402.55 PRESSURE GAUGES

- The Contractor shall install one (1) diaphragm protected, glycerine oil filled, direct mounting, bottom connection pressure gauge complying with AS 1349 per centrifugal pump installation. Cases shall be fabricated from stainless steel complying with AS 1449 or bronze. The protective diaphragm shall be suitable for dismantling for cleaning without affecting the accuracy of the gauge.
- 2. The gauge face shall be 100mm in diameter and calibrated in metres head of water. The gauge shall accurately indicate the pump operating head and the pump no-flow head.
- 3. Each gauge shall be supplied with the nominally sized metric equivalent of three **Inclusions** (3) of the following bronze fittings: gate valve, union, nipple and reducing nipple.
- 4. Gauges and fittings shall be screwed into the pipe wall of ductile iron pipes, or pipe fittings, 150mm and larger. In pipework less than 150mm, gauges and fittings shall be screwed into a tapping band. On rising mains, where shown on the design plans, the Contractor shall install a ball valve to allow removal of the gauge.
- 5. The pressure gauge range for single or parallel pumps duty shall be 0 to 1.7 *Gauge Range* times the closed valve head of the pumps.

#### C402.56 VALVES

5.

- 1. The Contractor shall ensure that the valves supplied are compatible with the pipework such that proper sealing is provided between the pipe flanges and the valve. The concrete lining in pipework shall not be chipped away or reduced to provide clearance from the working parts of valves.
- 2. The Contractor shall ensure that valves are installed so as to facilitate **Installation** maintenance. The Contractor shall take into account the manufacturer's recommendations, the requirements shown on the design plans, the type of connection, and lubrication of connecting bolts.
- 3. Flanges shall comply with AS 4087 Figure B5.
- 4. The direction of closing for stop valves shall be in accordance with Table C402.1

Table C402.1 Valve Closing Directions			
Council Name/s	Direction of closing		
Lismore, Kyogle, Byron, Clarence Valley	Clockwise Closing		
Richmond Valley, Ballina	Anti-Clockwise Closing		

Valve Key

Flanges

Valve Closing Direction

6. The Contractor shall size "Tee" Key valve operators and hand wheels to operate the valves under all operating conditions throughout their full range with no greater than 180 Newtons applied to the ends of the key bar or the rim of the wheel.

All sluice valves shall be resilient seated fully FBN coated.

Valve Key Operators and Hand wheels

- Hand wheels shall display an embossed or engraved arrow, together with "open" and/or "close" corresponding to the valve operation.
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8.	respe	Tee" key operator per pump station, of suitable length for operating the ctive valve from the surface level, shall be provided for each size of valve ed in each pump station.	Provision of "Tee" Key		
9.	cast ir leaf sl	eturn valves shall be of the swing check type to AS 3578 or AS 4794 of on or steel body, cover and disc with bronze body and disc seat rings. The hall swing clear and provide an unobstructed waterway. Non return valves be FBN coated.	Non Return Valves		
10.		The body cover shall be located and sized to allow the valve flap to be removed and the seat to be inspected without removing the valve.			
11.	stainle	Each non-return valve shall have an extended spindle, minimum grade 316 stainless steel, fitted with an adjustable counterweight, together with a proximity switch to indicate a no-flow condition.			
12.	The n	The no flow switches shall have the following features: <b>No Flow</b>			
	(a)	Be of the eccentric cam operated limit switch type.	Switches		
	(b)	Have a minimum rating of 10 amps, 240 V AC, 50- Hz.			
	(c)	Be oil tight and dust proof to IP 65.			
	(d)	Be suitable for 25mm conduit entry.			
	(e)	Be mounted on rigid stainless steel complying with AS 1444 adjustable brackets. The brackets shall be free of sharp edges and exposed corners.			
13.	The knife gate valve shall be constructed in accordance with the following:		Knife Gate Valve		
	(a)	The design shall include an enclosed bonnet.			
	(b)	The spindle shall be of the non-rising type.			
	(c)	The direction of closing for stop valves shall be in accordance with Table C402.1			
	(d)	The gland around the spindle shall be adjustable or formed by a double O-ring.			
	(e)	Flange jointing shall be rubber O-rings.			
	(f)	Seating shall be achieved by flexible seats which shall be designed in a manner that will allow easy replacement. The material of the seat is to be nominated.			
	(g)	The valve shall be unidirectional fully stainless steel construction.			
14.		sembly bolts and nuts shall be fitted with fibre or nylon isolating washers to nt bimetallic corrosion where required.	lsolating Washers		
15.	secure	valve spindle shall be fitted with a cast steel or forged steel spindle guard ed to the valve spindle with a gun metal set screw or a hand-wheel secured spindle with gun metal set screw and washer.	Spindle Guard		
16.	Valve: 4087.	s shall be drilled and threaded, where required, in accordance with AS	Drilled and Threaded		

## C402.57 TESTING AND COMMISSIONING OF PUMP STATION

1.	The Contractor shall test and/or inspect all materials, equipment, installation and workmanship to prove compliance with the Specification requirements. The		Compliance
		sion to Council of satisfactory test results constitutes a <b>HOLD POINT</b> . proval of Council is required prior to the release of the hold point.	HP
2.	Tests a	and inspections shall comply with relevant Australian Standards.	Standards
3.		g shall include pre-commissioning, field testing and performance testing of art of the whole installation.	Testing
4.	and pro	mmissioning is the preparation of plant or equipment so that it is in a safe oper condition and ready for commissioning and operation. It includes all s of plant operation such as safety, electrical, mechanical and mentation.	Pre- Commissioning
5.	The Contractor shall conduct pre-commissioning in a logical sequence in accordance with the programme prepared by the Contractor and approved by Council.		Sequence
6.	equipm	ontractor shall prepare pre-commissioning record sheets for each item of nent to ensure results of tests are satisfactorily recorded and that all sary checks or tests have been performed.	Record Sheets
7.	Specifi	c requirements for pre-commissioning shall include, but are not limited to:	Requirements
	(a)	Initial charges of lubricant in addition to any special lubricant requirements for initial flushing or treatment of the system or for "running in".	
	(b)	Physical checks and tests such as completeness of assembly, rotational tests (including checking that the rotation of electrical motors is in the correct direction), alignment checks, balancing and vibration checks, temperature, pressure and flow measurements, clearances, belt alignment and tension, etc, depending on the type of equipment.	
	(c)	Electrical and instrument installation tests, including motor insulation tests and checking instruments against certified instruments and correcting as necessary.	
	(d)	Tests of the correct functioning of automatic and manual control and protection equipment, including simulating danger conditions, mal- operations or failures, to check that all instruments and controls function correctly. These tests shall also include adjusting instrument set points and alarm settings and proving correct operation of alarms.	
	(e)	Equipment and system operating tests. The Contractor shall certify compliance of each item and submit a signed copy to Council prior to commissioning.	
8.	Counci	ontractor shall carry out pre-commissioning tests to the satisfaction of il and shall record the results of the tests on the appropriate Pre- ssioning Record Sheet.	Recording
9.	Pre-co	ontractor shall furnish Council with one (1) signed copy of each completed mmissioning Record Sheet countersigned by Council's Representative tnessed the test.	Submission

- 10. Commissioning is the running of the plant and equipment to ensure flow through **Commissioning** the pumping system, carrying out any necessary testing and adjustments until it is ready and suitable for normal starting and running under service conditions.
- 11. The Contractor shall give Council five (5) working days notice of the Contractor's intention to undertake commissioning and supply to Council the copies of each of the pre-commissioning record sheets and three (3) copies of the operational and maintenance manuals at the time that notice of commissioning is given. This action constitutes a **WITNESS POINT**. Council shall advise at the time of notification by the Contractor whether the option to attend the commissioning is to be exercised.
- 12. The Contractor shall conduct commissioning in a logical sequence in accordance **Sequence** with a programme prepared by the Contractor and approved by Council.
- 13. Throughout commissioning the Contractor shall be responsible for the test **Responsibility** programme.

14.	The Contractor shall provide continuous supervision by personnel experienced in the operation of the equipment and shall have qualified personnel in attendance		
	to carry out all necessary adjustments and/or remedial work during the commissioning tests.		

15. The Contractor shall prepare, schedules, test record sheets and programmes for **Documentation** approval by Council prior to each stage of the overall commissioning.

16.	The Contractor shall carry out final testing and commissioning (min 1 day	Final Testing
	duration) of the electrical services in conjunction with the mechanical equipment	
	(e.g. pump, etc) including setting and adjustment of equipment in accordance	
	with MEW E101.	

17. The Contractor shall arrange for all testing, commissioning and any adjustments **Qualified** to be carried out by qualified personnel. **Personnel** 

### C402.58 PRACTICAL COMPLETION OF PUMP STATION

- 1. The Contractor shall fulfil the following requirements before the Certificate of **Certificate** Practical Completion is issued:
  - (a) Receipt by Council of a certificate of approval from the relevant statutory authorities.
  - (b) Pump station is in working order as demonstrated by the testing and commissioning.
  - (c) Approval by Council of operating and maintenance manuals.
  - (d) Receipt by Council of as-built design plans of the pump station.

#### C402.59 TELEMETRY

1. The Contractor shall make provision for equipment to link the pump station to the existing telemetry network as applicable. **Contractor's** 

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Notification

WP

Pumps

The pump station shall be capable of being operated automatically by control signals from the existing or proposed telemetry system. In addition, either one (1) or any combination of pumps may operate at any one (1) time by control signals from the telemetry system.

### C402.60 OPERATION AND MAINTENANCE MANUALS

- 1. Manuals shall contain the following information:
  - (a) Contractor's name, address and telephone number.
  - (b) Client's Contract number, job name.
  - (c) Pump station general arrangement drawing showing pumps, motors, valves, pipework, switchboard and electrical installation.
- 2. Manuals for pumps shall contain the following information:
  - (a) Manufacture.
  - (b) Type and model number.
  - (c) Serial number.
  - (d) Dimensioned general arrangement drawing of pump and motor.
  - (e) Sectional arrangement drawing with parts and list.
  - (f) Dimensioned sectional arrangements detailing:
    - (i) Maximum and minimum shaft/bearing clearance (radial)
    - (ii) Maximum and minimum impeller/bowl clearance (radial)
    - (iii) Maximum and minimum impeller/bowl clearance (axial)
    - (iv) Impeller/bowl wear rings.
    - (v) Motor/pump coupling type, make and model number.
    - (vi) Mechanical seals where applicable.
- 3. Manual for motors shall contain the following information:
  - (a) Manufacture.
  - (b) Type and model number.
  - (c) Serial number.
  - (d) Dimensioned general arrangement drawing.
  - (e) Sectional arrangement drawing for submersible motor power cabling where applicable.
  - (f) Gland sealing arrangement drawing for submersible motor power cabling where applicable.
  - (g) Cables where applicable.
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Motors

- (h) Terminal block arrangement drawing where applicable.
- 4. Manuals for valves shall contain a dimensioned sectional arrangement drawing **Valves** with parts and material list for all valves.
- 5. Manuals shall contain the following test curves:
  - (a) Pump witnessed test curves.
  - (b) Motor test curves.
  - (c) Motor torque/speed/efficiency characteristic curves.
- 6. The operation and maintenance manual shall include:
  - (a) Safe working procedures: For switching and isolating the supply and distribution system;
  - (b) Comprehensive description of operation, including flow charts detailing each operational activity (e.g. manual pump operation, routine test procedures);
  - (c) Maintenance procedures: Recommended maintenance periods and procedures;
  - (d) Tools: Particulars of maintenance equipment and tools provided, with instructions for their use.
  - (e) Equipment: A technical description of the equipment supplied, with diagrams and illustrations where appropriate;
  - (f) Dismantling: Where necessary, procedures for dismantling and reassembling equipment;
  - (g) Spare parts: A list of the spare parts provided.
- 7. Trouble shooting instructions shall be included for pumps, motors, valves and **Trouble** SCA. **Shooting**
- 8. Step by step procedures for dismantling and reassembly of pumps, motors and valves using any special tools shall be detailed together with step by step procedures for replacement of wearing parts such as bearing, seals, wear rings, etc.
- Council shall supply Council with a hard copy printout of the operation and maintenance manual, and a CD containing the word-processor file(s) (Microsoft Windows compatible format) for the document and a copy of the document in PDF format. One spare set of pump and motor tags shall be provided with the O&M manuals.

# CONSTRUCTION COMPLIANCE

## C402.61 WORK-AS-EXECUTED DETAILS

1. The Contractor shall submit Work-As-Executed Plans in accordance with Council *Main* requirements. *Requir* 

Main Requirements

**Test Curves** 

Operation and Maintenance

### C402.62 OPERATION AND MAINTENANCE MANUALS

The Contractor shall submit to Council all operation and maintenance manuals at the time of commissioning or when handed over for Council's operation. All operation and maintenance manuals must be included in the subdivision works compliance certificate attachments.

# SPECIAL REQUIREMENTS

### C402.63 CCTV INSPECTION

### C402.64 WHEN IS A CCTV INSPECTION REQUIRED

- Unless advised otherwise by Council, a CCTV inspection to verify the internal condition of all sewer pipe infrastructure is required to be undertaken after backfilling and after completion of all subdivision works and passage of construction equipment above the mains (normally around the time of practical completion of the subdivision works).
- 2. A CCTV inspection of all sewer pipe infrastructure is required to be undertaken on or immediately after the expiration of the defects liability (maintenance) period.
- 3. Additional CCTV inspections are required of any remediation / repair works undertaken to the sewer infrastructure, or as directed by Council and/or Council to demonstrate that the standard of the infrastructure is acceptable. Other CCTV Inspections (as required)
- 4. The CCTV assessment shall also include any existing sewer infrastructure that is to be utilised within the design. Council will advise if any repair / upgrades to the infrastructure existing system are required.

## C402.65 WHAT IS TO BE INSPECTED

- 1. All sewer networks are to be CCTV inspected.
- 2. Pipes shall be inspected and reported on the following:
  - a. Horizontal alignment
  - b. Vertical alignment
  - c. Cracks and defects
  - d. Pipe joints
  - e. Joints to manholes and other pipes
  - f. Ovality

Assessment Criteria

#### C402.66 PRE INSPECTION CRITERIA

1. All pipes are to be inspected upon delivery. A "No cracks policy" is to be adopted, giving the site supervisor the authority to reject any pipe with a crack **policy** when delivered.

#### C402.67 INSPECTION CRITERIA

- 1. CCTV surveys are to be undertaken using a camera with the ability to capture footage in colour and pan and tilt 360°.
- 2. All pipes must be free of debris and silt at the time of inspection.
- 3. The pipeline shall be assessed at the following speeds

Conduit DiameterAllowable Camera SpeedDia. < 200mm</td>0.1m/s \*200mm ≤ Dia. < 300mm</td>0.15m/s \*Dia. ≥ 300mm0.2m/s \*\* - Or as agreed by Council

- 4. The camera must stop perpendicular at all joints but does not need to pan 360°. **Stop at joints** However, particular attention should be paid to any infiltration at joints and connections.
- 5. The camera must stop perpendicular to all cracks, defects, junctions and **Stop and Pan** manholes, and pan 360°.

### C402.68 ACCEPTANCE CRITERIA

1.	The pipeline will be acceptable if Council is satisfied that the CCTV inspection does not reveal any defects that would constitute a departure from this specification or any other relevant specification.	Acceptance of CCTV Inspection
2.	Sections of the pipeline that fail the ovality test are to be excavated and the trench and embedment replaced. Where this reveals that any pipes are crushed or creased, these are to be replaced.	Failed Items
3.	Any pipes that are crushed, split or creased are to be replaced.	Defective Pipes

### C402.69 SUBMISSION

1. The Contractor must submit both a hardcopy report and an electronic report (submitted in CD or DVD medium in a format suitable to Council) of the CCTV inspection. The CCTV Inspection Report is a pre-requisite for issue of an *Off Defects Liability Compliance Certificate*.

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Speed

- 2. The reports must
  - a. Specify the date of the inspection
  - b. Specify location (including Street Name and number)
  - c. Specify details of the reach being inspected (including line and structure numbers)
  - d. Provide footage in colour
  - e. Identify all faults, features and connections in the pipeline.
  - f. Clearly show chainage along the pipeline
  - g. Suggest appropriate remediation measures, as required.

## C402.70 IF REMEDIATION WORKS ARE REQUIRED

- 1. Any defects identified by the inspection must be repaired or replaced in accordance with the provisions of this Specification, or as directed by Council.
- 2. All costs associated with the CCTV inspection and rectification works shall be **Costs** borne by the Contractor.
- 3. A follow-up CCTV assessment is required of any repaired or replaced infrastructure, to demonstrate that the remediation measures undertaken are satisfactory to Council.
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- C402.72 RESERVED

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